MEMBER & ABBREVIATION LEGEND

GBT = GRADE BEAM TRANSITION @ BOTTOM ON 45° ANGLE.
GBS = GRADE BEAM STEP – RE: DETAIL 14S7.
U.N.O. = UNLESS NOTED OTHERWISE.
TYP. = TYPICAL.
T.O.G.B. = TOP OF GRADE BEAM.
T6 = TS 6x6x⅜ COLUMN W/ BASE PLATE TYPE C.
CT = GALVANIZED W10x33 @ +5'–0" – WELD TO EMBED PLATE. TOUCH UP WELDS W/ GALVANIZING REPAIR PAINT.
(#) = VERIFY ALL DIMENSIONS W/ EQUIPMENT SUPPLIER.
= TOP OF HATCHED GRADE BEAM @ EL. –3'–2".
= TOP OF HATCHED GRADE BEAM @ EL. –3'–11".

NOTE:
BOTTOM OF ALL COLUMN BASE PLATES @ EL. –0'–6½" W/ 1½" NON–SHRINK GROUT BELOW PLATE (TYP. – U.N.O.) – RE: 1S6.

NOTE A:
WIDEN GRADE BEAM TO 3'–0" WIDE & REINFORCE W/ 4 #9 T&B CONTINUOUS & #4 ☐ TIES @ 10" O.C. & SLAB DOWELS.

NOTE B:
ADD #5 @ 32" O.C. FULL HEIGHT OF CMU WALL IN CONCRETE FILLED CMU CELL. EXTEND 20" INTO GRADE BEAM & HOOK. USE 36" MIN. LAP SPLICES. APPLIES AT THIS CURVED GRADE BEAM ONLY.

NOTE C:
REINFORCE GRADE BEAM SIMILAR TO DETAIL 3S7. TOP OF GRADE BEAM @ EL. –2'–0".

NOTE D:
2'–8"x4'–4" PEDESTAL. REINFORCE W/ 12 #5 VERTICAL & 4 #4 ☐ TIES. TOP OF PEDESTAL @ EL. –1'–4".

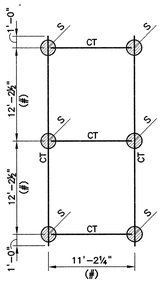
NOTE E:
3'–2"x3'–2" PEDESTAL. REINFORCE W/ 12 #5 VERTICAL & 4 #4 ☐ TIES.

NOTE F:
EXTEND AND WIDEN GRADE BEAM. REINFORCE W/ 3 #9 T&B CONTINUOUS & #4 ☐ TIES @ 10" O.C. TOP OF GRADE BEAM @ EL. –1'–4" – BEND BARS FOR CONTINUITY.

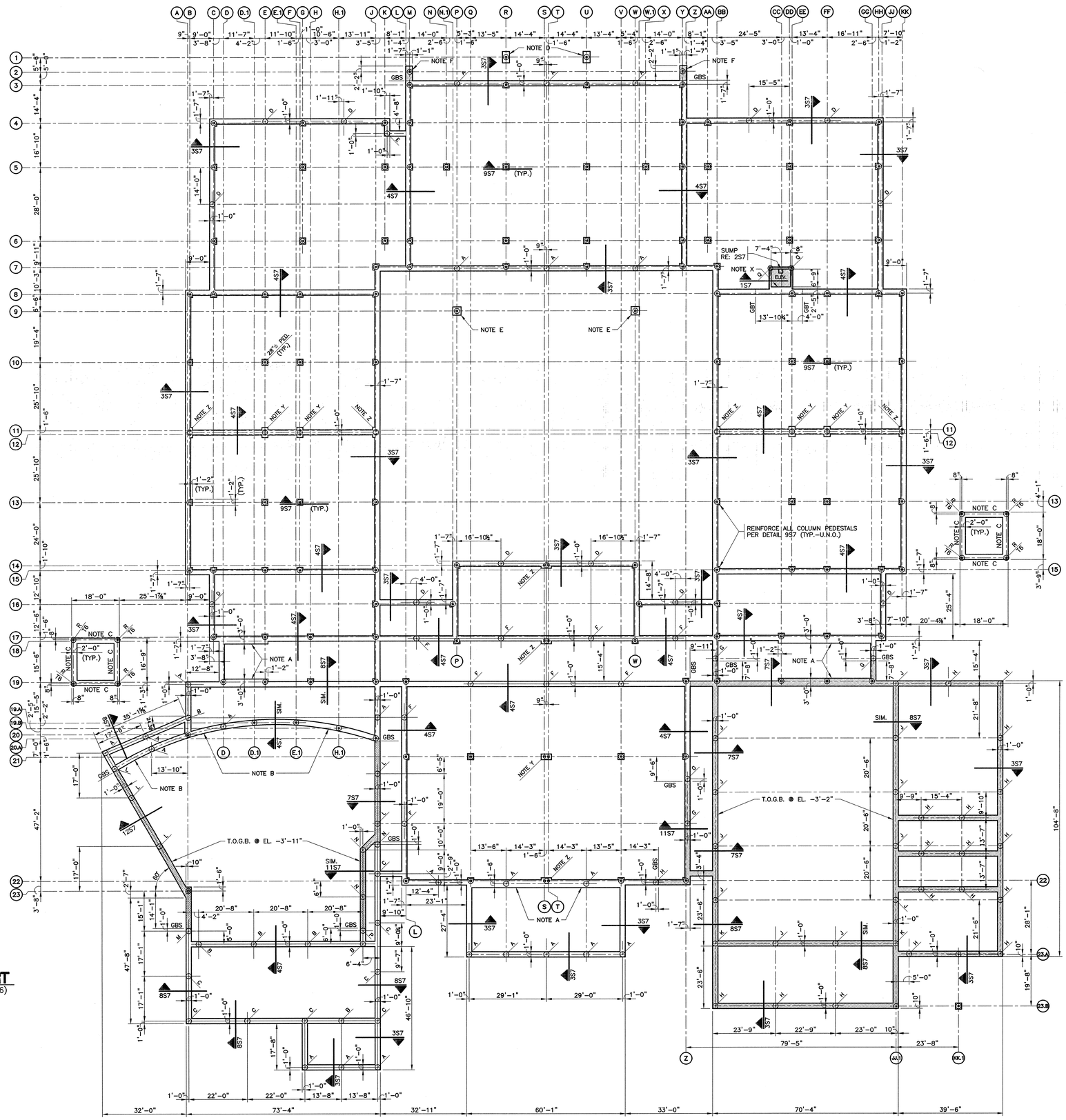
NOTE X:
WIDEN GRADE BEAM AT ELEVATOR & ADD #5 @ 10" O.C.E.W. AT PIT SIDE ONLY.

NOTE Y:
2'–4"x3'–10" PEDESTAL REINFORCED W/ 12 #5 VERTICALS & 4 #4 ☐ TIES.

NOTE Z:
2'–9"x3'–10" PEDESTAL MADE FROM GRADE BEAM FLAREOUT REINFORCED W/ 12 #5 VERTICAL & 4 #4 ☐ TIES.



2 COOLING TOWER SUPPORT
1/8" = 1'–0" (RE: 1S6)



1 FOUNDATION PLAN
1/16" = 1'–0" (RE: 1S6)
(ELEV. 0'–0" = SITE EL. 49.0')

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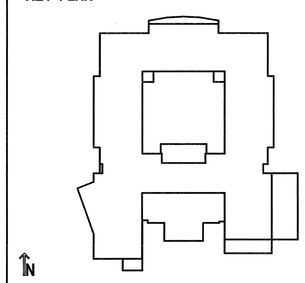
KEY PLAN



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FOUNDATION PLAN
COOLING TOWER SUPPORT

STATE OF LOUISIANA
DANNY J. DEVILLE
REG. NO. 20075
REGISTERED PROFESSIONAL ENGINEER
CIVIL ENGINEERING

PROJECT NO.	03048
DRAWN BY	G.P.G.
CHECKED BY	D.J.D.
DATE	October 15, 2004
SHEET NO..	S1
OF	

MEMBER & ABBREVIATION LEGEND

P/C = PRECAST CONCRETE.

U.N.O. = UNLESS NOTED OTHERWISE.

TYP. = TYPICAL.

NOTE:

SEE ARCH. DRAWINGS FOR ALL DAPS, SLOPES, DEPRESSIONS, STAIRS, RAMPS, ETC.

NOTE:

CONTRACTOR MAY RELOCATE SLAB CONSTRUCTION JOINTS (C.J.) TO FACILITATE EACH DAYS CONCRETE PLACEMENT.

NOTE:

PROVIDE TS 6x6x¼ GUIDE RAIL COLUMN SUPPORTS @ VERTICAL RAILS OF ELEVATOR AS REQUIRED – ATTACH TO STRUCTURE AS REQUIRED. ATTACH TO SLAB W/ 2–¾" ø ANCHOR BOLTS.

NOTE A:

PROVIDE CONCRETE PLYNTH TO EL. –0'–8" – MATCH ARCH. PILASTER PROFILE. FILL PILASTER W/ 3000 PSI CONCRETE FULL HEIGHT.

NOTE B:

6" SIDE WALLS W/ #4 @ 10" O.C.E.W.

NOTE C:

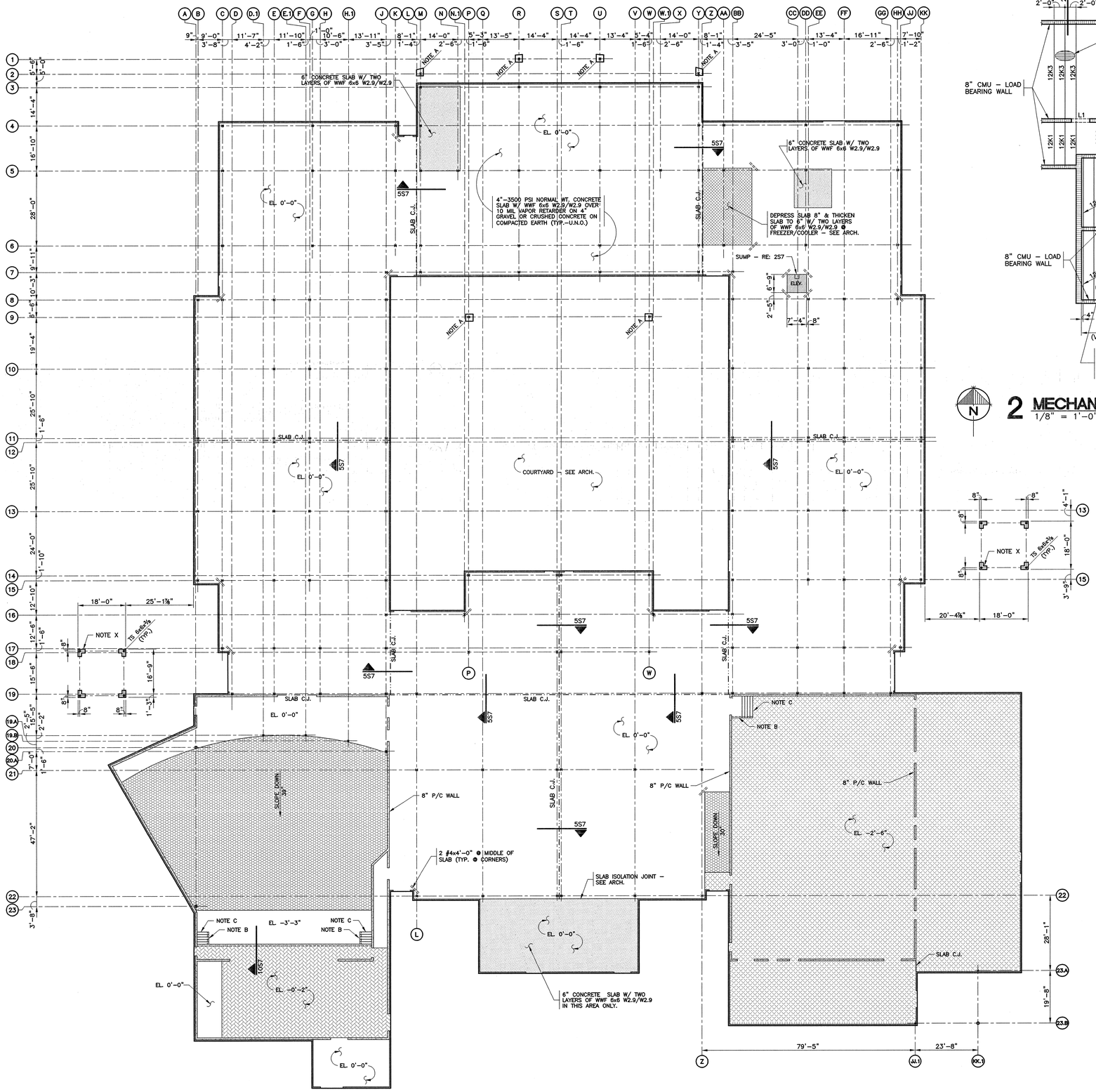
STEPS ON SAND FILL – SEE ARCH. – REINFORCE EACH NOSE W/ #4.

NOTE:

SEE ARCH. DETAILS & SPECS FOR WATERPROOFING REQUIREMENTS @ DEPRESSED SLABS & WALLS IN AUDITORIUM & GYMNASIUM.

NOTE X:

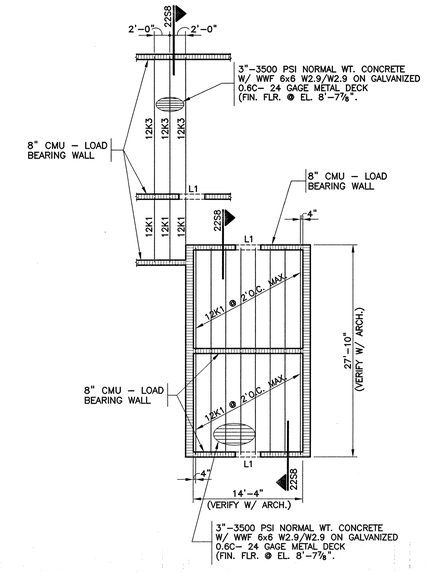
PROVIDE L–SHAPED CONCRETE PLYNTH TO EL. –1'–4". MATCH ARCH. BRICK PILASTER PROFILE.



1 SLAB PLAN

1/16" = 1'–0"

(ELEV. 0'–0" = SITE EL. 49.0')



2 MECHANICAL MEZZANINE FRAMING PLAN

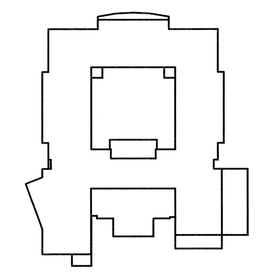
1/8" = 1'–0" (ABOVE ROOMS 305 & 308)

NOTE:

SEE ARCH. FOR LOCATION & DIMENSIONS FOR MEZZANINE AND ALL GUARDRAIL INFORMATION.

L1 = 8" DEEP CONCRETE FILLED U–BLOCK W/ 2 #5 BOTTOM @ HEAD OF OPENING. 8" BEARING @ ENDS.

KEY PLAN



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SLAB PLAN
MECHANICAL MEZZANINE FRAMING PLAN

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PROJECT NO.	03048
DRAWN BY	G.P.G.
CHECKED BY	D.J.D.
DATE	October 15, 2004
SHEET NO..	S2
	OF

MEMBER & ABBREVIATION LEGEND

A = PROVIDE TS 4x4x¼ CONTINUOUS WELDED TO BOTTOM FLANGE OF BEAMS AS REQUIRED FOR HOOD SUPPORT – SEE KITCHEN EQUIPMENT DRAWINGS.

B = W10x12.

C = W14x22 – ATTACH TO COLUMN WITH FULL PENN. BUTT WELDS – CUT BOTTOM FLANGE & ADD PLATE PER DETAIL 11S8 – SEE ARCH.

D = W14x22 – CUT BOTTOM FLANGE & ADD PLATE PER DETAIL 11S8 – SEE ARCH.

BB = REINFORCED CONCRETE BRICK BEAM W/ 3000 PSI CONCRETE FILL PER ARCH. DETAILS. REINFORCE WITH 2 #8 T&B CONTINUOUS & #3 ⊓ TIES @ 10" O.C. WELD REBARS TO PLATE ON STEEL COLUMN. ALSO PROVIDE GALVANIZED ⅜" TEE BOTTOM PLATE 9" x 15" BEAR 8" ON FILLED PILASTER @ ENDS.

[1] CUT BOTTOM FLANGE & ADD PLATE PER DETAIL 11S8 – SEE ARCH.

[2] PEICE OF W12x14 – CONNECT TO COLUMN & BEAM.

[3] CONNECT BEAM TO ½" EMBED PLATE IN P/C CONCRETE PANEL. SERVICE DESIGN SHEAR EQUAL TO 20K.

[4] PROVIDE #5 @ 4' O.C. IN CONCRETE FILLED CELL FULL HEIGHT OF WALL.

[5] FILL PILASTER WITH 3000 PSI CONCRETE FULL HEIGHT.

[6] CONFIRM STAIR OPENING DIMENSIONS PRIOR TO SUBMITTING SHOP DRAWINGS. SEE GENERAL NOTES FOR STAIR CONSTRUCTION.

[7] CONNECT TOP FLANGE OF BEAM TO P/C WALL W/ PLATE ⅜"x4"x8" @ 3' O.C. WELDED TO EMBED PLATE IN P/C WALL & BEAM ALL AROUND.

U.N.O. = UNLESS NOTED OTHERWISE.

TYP. = TYPICAL.

T.O.S. = TOP OF STEEL.

P/C = PRECAST CONCRETE.

NOTE C:

REPRESENTS THE TOTAL NUMBER OF ¾"ø x 3½" SHEAR STUDS TO BE EQUALLY SPACED ON THE BEAM BETWEEN ITS SUPPORT POINTS – SEE GENERAL NOTES.

NOTE:

ALL BEAMS & GIRDERS CONTAINING SHEAR STUDS IN FLOOR AREAS SHALL BE SHORED @ MID-SPAN. (MIN. 6x6 WOOD POST OR STEEL SHORE). UNTIL CONCRETE FILL ON METAL DECK REACHES A STRENGTH OF 2625 PSI.

NOTE:

ALL STEEL SHALL BE ASTM A572–GRADE 50 OR ASTM A992.

NOTE:

PROVIDE FRAMED OPENING AROUND ALL ROOF OPENINGS LARGER THAN 6"ø AND AT ALL ROOF DRAINS PER DETAIL 8S8.

NOTE:

PROVIDE ANGLE 5x5x5/16 FLOOR EDGE CLOSURE ANGLES AT INTERIOR OPENINGS, STAIRS, ELEVATORS, ETC.

NOTE:

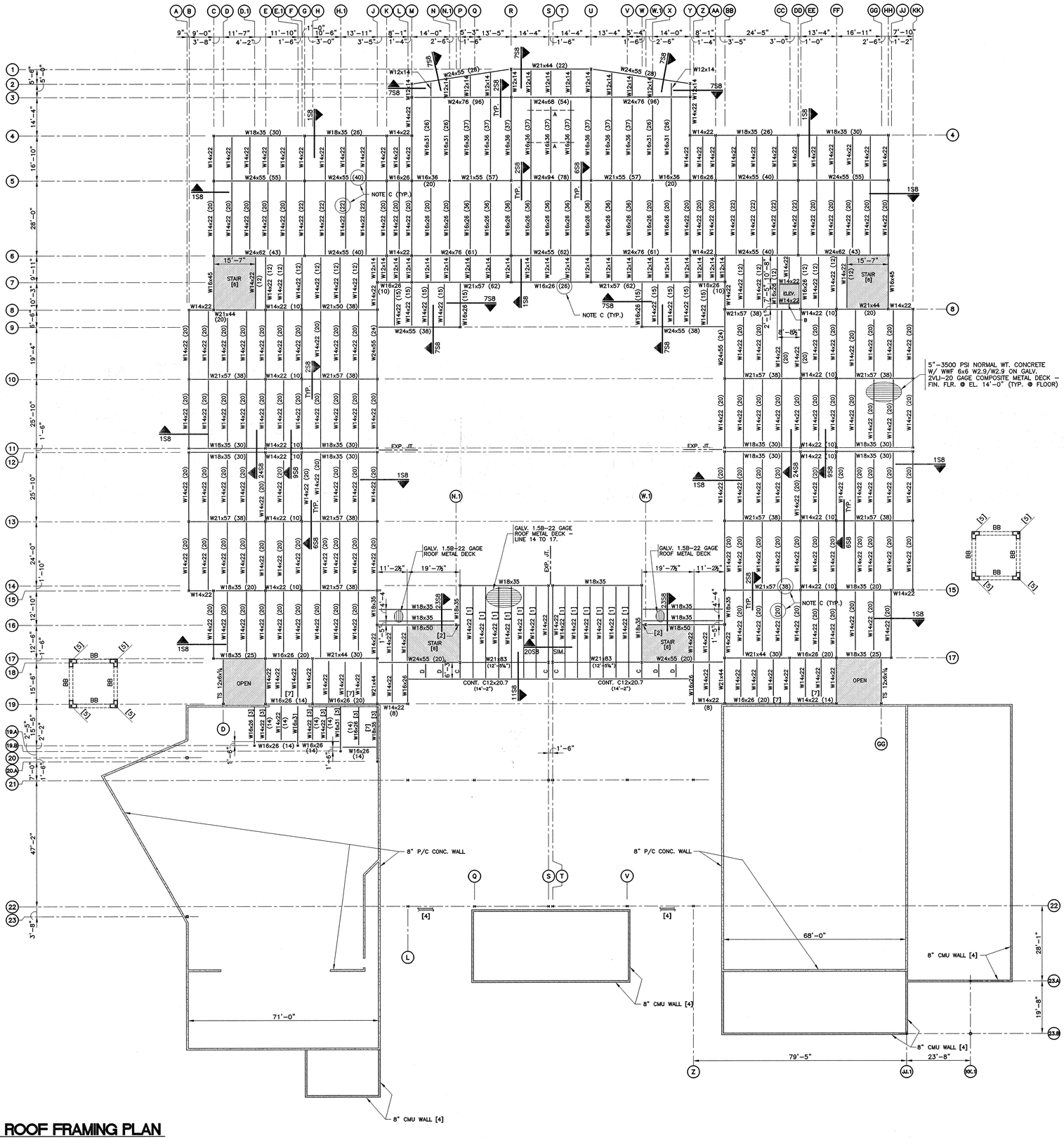
PROVIDE C5x6.7 FRAME AROUND ALL FLOOR OPENINGS LARGER THAN 6"ø.

NOTE:

EQUALLY SPACE BEAMS – U.N.O.

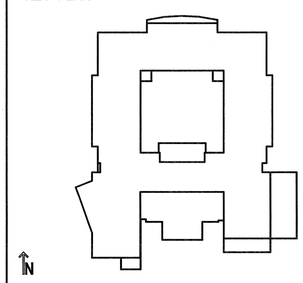
NOTE:

PROVIDE TS 6x6x¼ GUIDE RAIL COLUMN SUPPORTS @ VERTICAL RAILS OF ELEVATOR AS REQUIRED – ATTACH TO STRUCTURE AS REQUIRED.



1 **SECOND FLOOR AND LOW ROOF FRAMING PLAN**
1/16" = 1'–0" (T.O.S. @ EL. 13'–7" – U.N.O.)

KEY PLAN



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70806
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tel 225.387.4414
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SECOND FLOOR AND LOW ROOF FRAMING PLAN

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STATE OF LOUISIANA
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DRAWN BY	G.P.G.
CHECKED BY	D.J.D.
DATE	October 15, 2004
SHEET NO..	S3
	OF

MEMBER & ABBREVIATION LEGEND

U.N.O. = UNLESS NOTED OTHERWISE.

TYP. = TYPICAL.

T.O.S. = TOP OF STEEL.

P/C = PRECAST CONCRETE.

C.W. = SUSPENDED CATWALK – SEE DETAIL 18S8.

S.J. – STRUT JOIST – CONNECT BOTTOM CHORD TO COLUMN.

(A) TS 6x3x¼ STUB – WELD TO COLUMN ALL AROUND – LOCATE IN PLANE OF 8" RAFTERS.

[1] PROVIDE #5 @ 4' O.C. IN CONCRETE FILLED CELL FULL HEIGHT OF WALL

[2] CONNECT TOP FLANGE OF BEAM/JOIST TO P/C WALL W/ PLATE ⅜"x4"x8" @ 3' O.C. WELDED TO EMBED PLATE IN P/C WALL & BEAM/JOIST ALL AROUND.

[3] CONNECT BEAM TO ½" EMBED PLATE IN P/C CONCRETE PANEL. SERVICE DESIGN SHEAR EQUAL TO 20K.

[4] BEAM SLOPES.

NOTE A:

CONTINUOUS ANGLE 5x5x⅜ WELDED TO EMBED PLATES IN P/C WALL @ 2' O.C. – SEE DETAIL 19S8.

NOTE B:

CONTINUOUS W6x9 @ 6' O.C.± WELDED TO BOTTOM CHORD OF EACH JOIST. LOCATE AT NEAREST JOIST BOTTOM CHORD PANEL POINT. (11 ROWS REQ'D)

NOTE C:

PRECAST CONCRETE WALL SUPPLIER SHALL DESIGN WALL SYSTEM FOR ALL LOADS IMPOSED BY BASKETBALL GOAL ASSEMBLY MOUNTED TO WALL. BASKETBALL GOAL LOADS ARE ASSUMED TO NOT BE TRANSFERRED TO ANY ROOF JOIST MEMBERS.

NOTE D:

PROVIDE 2-BAYS OF X-BRACING ANGLE 3x3x¼ AND ATTACH BOTTOM CHORD OF JOIST TO P/C WALL AT THIS LOCATION.

NOTE:

ALL STEEL SHALL BE ASTM A572-GRADE 50 OR ASTM A992.

NOTE:

PROVIDE FRAMED OPENING AROUND ALL ROOF OPENINGS LARGER THAN 6"ø AND AT ALL ROOF DRAINS PER DETAIL 8S8.

NOTE:

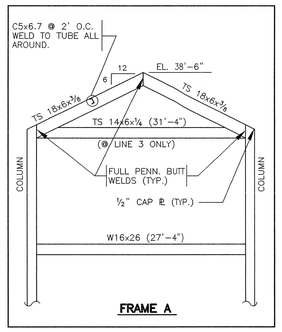
EQUALLY SPACE JOISTS – U.N.O.

NOTE:

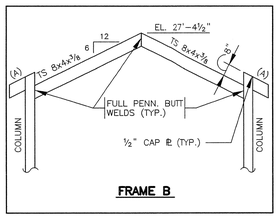
DESIGN ROOF JOISTS FOR A NET UPLIFT OF 20 PSF.

NOTE:

PRECAST CONCRETE WALL SUPPLIER SHALL DESIGN WALLS FOR ALL APPROPRIATE GRAVITY, WIND, & EARTHQUAKE LOADS PER IBC-2000. SEE GENERAL NOTES SHEET S7.



FRAME A



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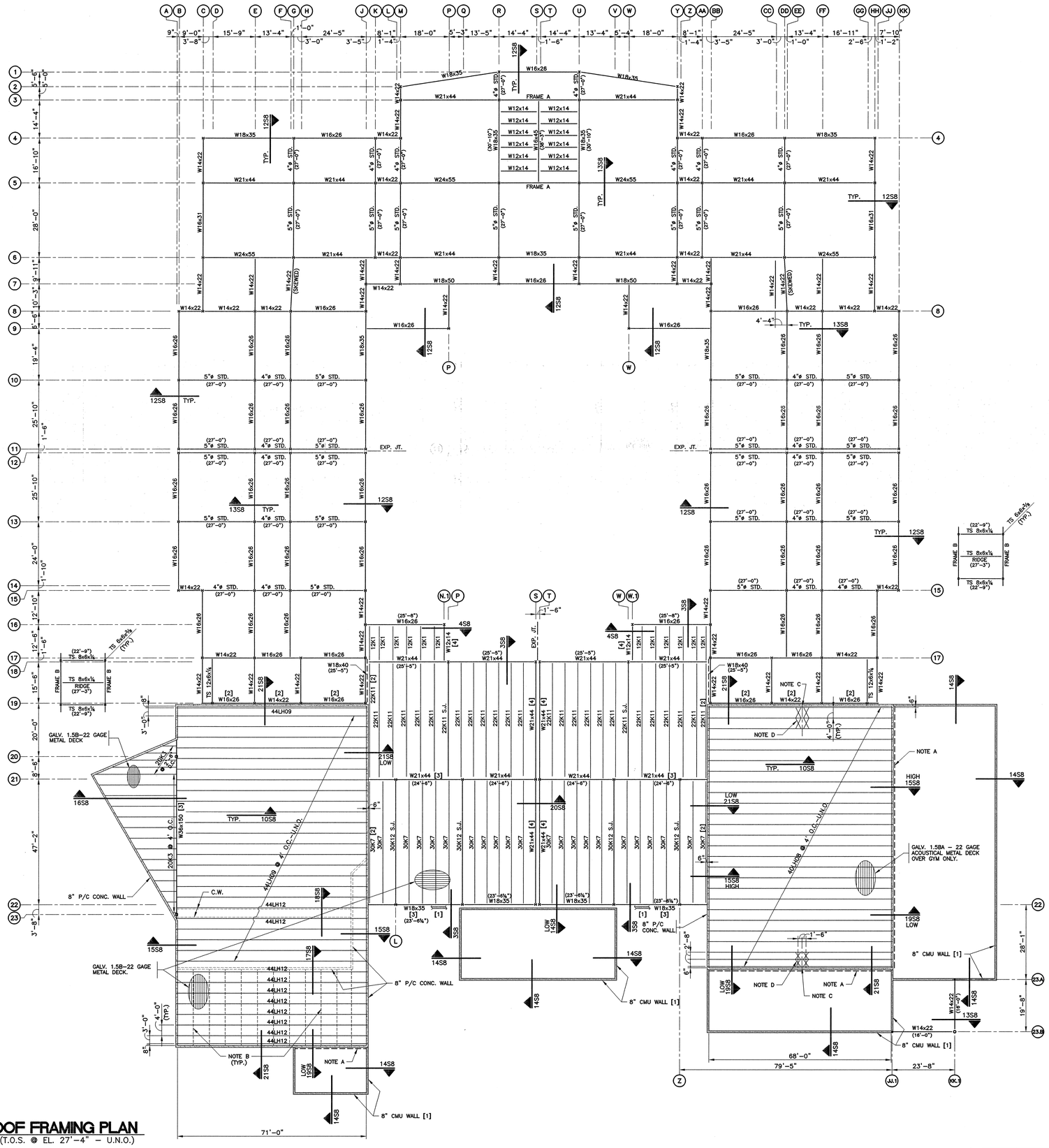
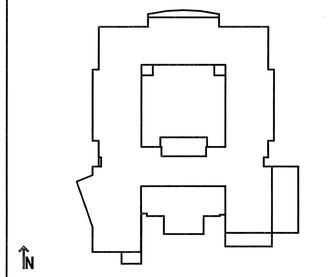


1 ROOF SUPPORT AND HIGH ROOF FRAMING PLAN

1/16" = 1'-0" (T.O.S. @ EL. 27'-4" – U.N.O.)

KEY PLAN



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ROOF SUPPORT AND HIGH ROOF FRAMING PLAN

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REG. NO. 29075
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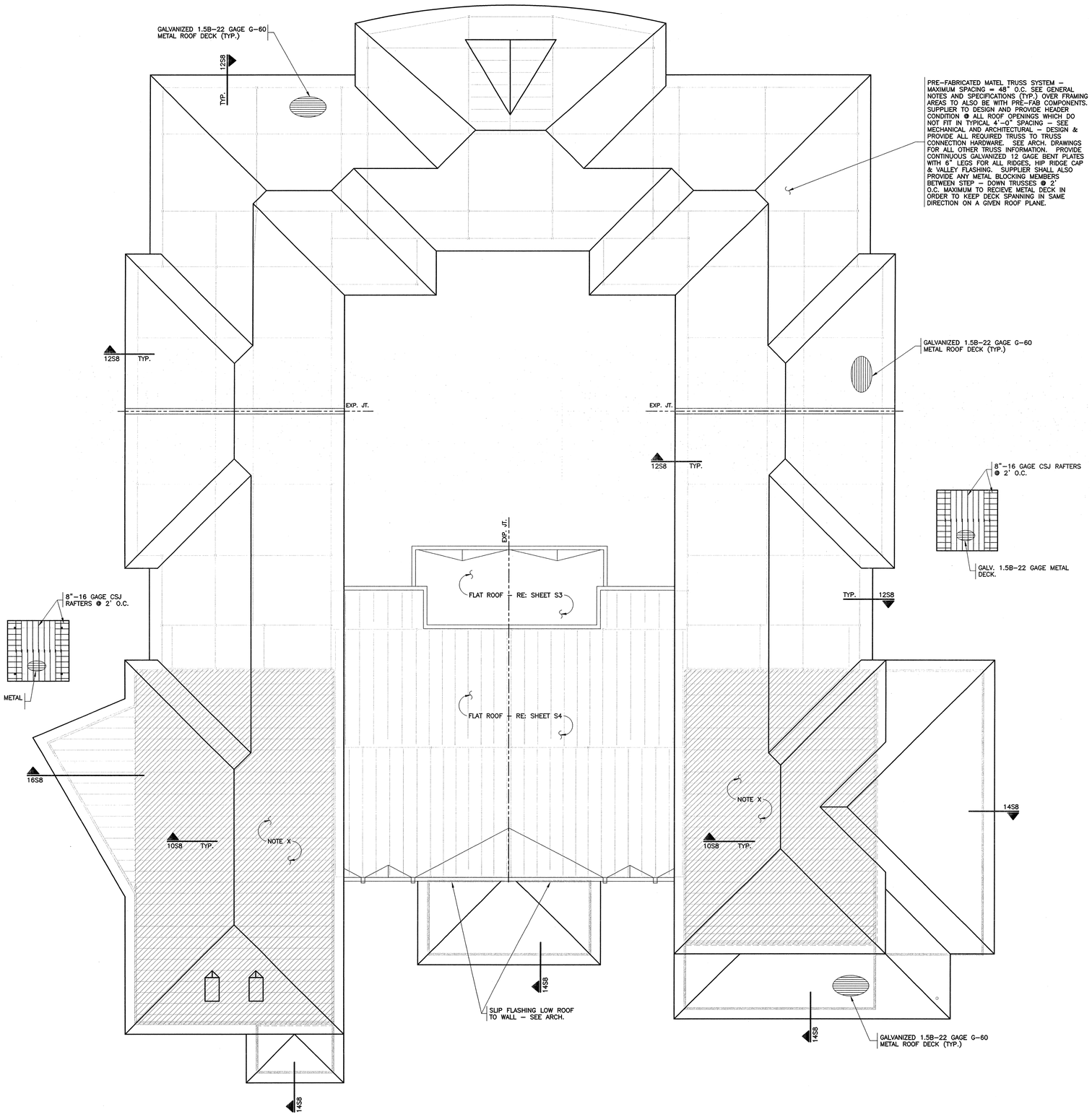
NOTE:

A METAL TRUSS SHALL BE POSITIONED AT ALL HIP & RIDGE INTERSECTION POINTS AND VALLEY & RIDGE INTERSECTION POINTS. HIP GIRDER SET BACK IS TO BE DETERMINED BY TRUSS MANUFACTURER.

EXP. JT. = ¾" EXPANSION JOINT – PROVIDE METAL TRUSS 3" EACH SIDE OF JOINT AND GAP DECK ¾".

NOTE X:

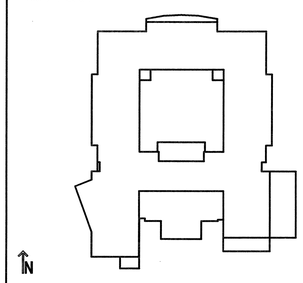
METAL TRUSSES IN THIS AREA WHICH SPAN PARALLEL TO SUPPORTING BAR JOIST BELOW SHALL ALIGN WITH BAR JOIST BELOW. CONNECT TRUSSES TO BAR JOIST PER DETAIL 10S8 (TYP.) TRUSSES NOT RUNNING PARALLEL TO BAR JOISTS SHALL BE CONNECTED TO EACH BAR JOIST THEY CROSS PER DETAIL 10S8.



1 ROOF FRAMING PLAN

1/16" = 1'-0"

KEY PLAN



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fax 225.387.4695

ROOF FRAMING PLAN

STATE OF LOUISIANA
DANNY J. DEVILLE
REG. NO. 20075
REGISTERED PROFESSIONAL ENGINEER IN CIVIL ENGINEERING

PROJECT NO. 03048	SHEET NO.. S5
DRAWN BY G.P.G.	
CHECKED BY D.J.D.	
DATE October 15, 2004	____ OF ____

McKEE & DEVILLE CONSULTING ENGINEERS INC.
5131 PAYNE DRIVE BATON ROUGE, LA. 70809 TEL. (225)767-7841
JOB NO. 203079 © 2004

(#) CONTRACTOR MAY USE SIZES LARGER THAN THESE SPECIFIED AT NO ADDITIONAL COST TO OWNER.
(✳) HOOK VERTICAL BARS @ TOP – BARS ON 12" GAGE EACH WAY @ 4 BAR PATTERN.
($) SHARES FOOTING WITH ADJACENT COLUMN.

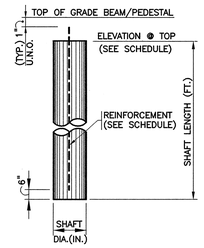
1 COLUMN AND DRILLED FOOTING SCHEDULE

(RE: 1S1)
(EL. 0'–0" = SITE EL. 49.0')

NOTE A: CENTER ON GRADE BEAM –U.N.O.

LOCATION/MARK	A–20	A–23	B–8	B–10	B–11	B–12	B–13	B–15	C–4	C–5	C–6	C–8	C–15	C–17	D–19	D.1–19.A	E–8	E–10	E–11
ELEV. @ TOP OF COLUMN	◄																		
CAP PLATE SIZE (INCHES)	◄																		
COLUMN SIZE (ASTM A992)	TS10x10x⅜	TS10x10x⅜	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	W8x40	TS6x6x¼	W8x31	W8x35	W8x31
BASE PLATE TYPE (RE: 1S7)	B	B	A	A	A	A	A	A	A	A	A	A	A	A	A	C	A	A	A
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–3'–9½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"
ELEV. @ TOP OF DR. FTG.	–2'–8"	–5'–11"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"
SHAFT DIAMETER (INCHES) (#)	36	36	30	36	42	—	36	30	30	36	36	36	36	30	30	30	30	36	36
SHAFT LENGTH (FEET)	40	40	35	45	45	—	45	35	35	40	40	35	35	35	35	35	35	35	35
VERTICAL REINF. (FULL HEIGHT)	4 #9	4 #9	1 #9	1 #9	4 #9	—	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	4 #9
TIES □	#4@12" □	#4@12" □	—	—	#4@12" □	—	—	—	—	—	—	—	—	—	—	—	—	—	#4@12" □
REMARKS	(✳)	(✳)	—	—	(✳)	($)	—	—	—	—	—	—	—	—	—	(✳)	—	—	(✳)

LOCATION/MARK	E–12	E–13	E–15	E–17	E–19	E.1–19.A	F–8	F–10	F–11	F–12	F–13	F–15	G–4	G–5	G–6	H–17	H–19	H.1–19.B	J–7	J–8	J–10	J–11
ELEV. @ TOP OF COLUMN	(SEE PLAN AND DETAILS)																					►
CAP PLATE SIZE (INCHES)	(½" x COLUMN SIZE – TYP. U.N.O.)																					►
COLUMN SIZE (ASTM A992)	W8x31	W8x31	W8x31	W8x31	W8x31	TS6x6x¼	W8x35	W8x35	W8x31	W8x31	W8x35	W8x35	W8x31	W8x40	W8x40	W8x35	W8x31	TS6x6x¼	W8x31	W8x40	W8x35	W8x31
BASE PLATE TYPE (RE: 1S7)	A	A	A	A	A	C	A	A	A	A	A	A	A	A	A	A	A	C	A	A	A	A
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"
ELEV. @ TOP OF DR. FTG.	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"
SHAFT DIAMETER (INCHES) (#)	—	36	30	36	36	30	36	36	36	—	36	36	36	36	36	36	36	30	30	42	42	42
SHAFT LENGTH (FEET)	—	35	35	35	35	35	35	35	35	—	35	35	35	40	40	35	45	35	35	45	45	45
VERTICAL REINF. (FULL HEIGHT)	—	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	4 #9	—	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	4 #9
TIES □	—	—	—	—	—	—	—	—	#4@12" □	—	—	—	—	—	—	—	—	—	—	—	—	#4@12" □
REMARKS	($)	—	—	—	—	(✳)	—	—	(✳)	($)	—	—	—	—	—	—	—	(✳)	—	—	—	(✳)



TYPICAL DRILLED FOOTING

(#) CONTRACTOR MAY USE SIZES LARGER THAN THESE SPECIFIED AT NO ADDITIONAL COST TO OWNER.
(✳) HOOK VERTICAL BARS @ TOP – BARS ON 12" GAGE EACH WAY @ 4 BAR PATTERN.
($) SHARES FOOTING WITH ADJACENT COLUMN.

COLUMN AND DRILLED FOOTING SCHEDULE

(RE: 1S1)
(EL. 0'–0" = SITE EL. 49.0')

NOTE A: CENTER ON GRADE BEAM –U.N.O.

LOCATION/MARK	J–12	J–13	J–15	J–16	J–17	J–19	J–20.A	K–4	K–5	K–6	L–19	L–21	L–22	M–2	M–3	M–4	M–5	M–6	M–7
ELEV. @ TOP OF COLUMN	◄																		
CAP PLATE SIZE (INCHES)	◄																		
COLUMN SIZE (ASTM A992)	W8x31	W8x31	W8x31	W8x31	W8x35	W8x31	TS6x6x¼	W8x31	W8x35	W8x35	W8x31	W8x35	W8x31	W8x31	W8x31	W8x31	W8x31	W8x35	W8x35
BASE PLATE TYPE (RE: 1S7)	A	A	A	A	A	A	C	A	A	A	A	A	A	A	A	A	A	A	A
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–1'–2½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"
ELEV. @ TOP OF DR. FTG.	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–5'–11"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–3'–4"	–3'–4"	–2'–8"	–2'–8"	–2'–8"	–2'–8"
SHAFT DIAMETER (INCHES) (#)	—	36	36	36	36	36	36	30	30	30	24	30	30	24	36	30	30	30	36
SHAFT LENGTH (FEET)	—	45	40	35	35	45	35	35	35	35	35	35	35	35	40	35	40	40	40
VERTICAL REINF. (FULL HEIGHT)	—	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	2 #9	1 #9	1 #9	1 #9	1 #9	1 #9
TIES □	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
REMARKS	($)	—	—	—	—	—	(✳)	—	—	—	—	—	—	(✳)	—	—	—	—	—

LOCATION/MARK	N–5	N.1–16	P–9	P–14	P–18	Q–21	Q–22	R–1	R–3	R–5	R–6	R–7	S–14	S–18	S–21	S–22	T–14	T–18	T–21	T–22	U–1	U–3
ELEV. @ TOP OF COLUMN	(SEE PLAN AND DETAILS)																					►
CAP PLATE SIZE (INCHES)	(½" x COLUMN SIZE – TYP. U.N.O.)																					►
COLUMN SIZE (ASTM A992)	W8x40	W8x40	W8x35	W8x31	W8x40	W8x40	W8x35	W8x35	W8x58	W8x58	W8x48	W8x35	W8x31	W8x40	W8x35	W8x31	W8x31	W8x40	W8x35	W8x31	W8x35	W8x58
BASE PLATE TYPE (RE: 1S7)	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–1'–2½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–1'–2½"	–1'–2½"	–0'–6½"
ELEV. @ TOP OF DR. FTG.	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–3'–4"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–3'–4"	–2'–8"
SHAFT DIAMETER (INCHES) (#)	30	30	30	30	36	30	36	30	42	42	36	36	36	36	30	36	—	—	—	—	30	42
SHAFT LENGTH (FEET)	40	35	35	35	35	35	40	40	50	50	45	40	35	35	35	35	—	—	—	—	40	50
VERTICAL REINF. (FULL HEIGHT)	1 #9	1 #9	2 #9	1 #9	1 #9	2 #9	1 #9	4 #9	1 #9	1 #9	1 #9	1 #9	4 #9	4 #9	4 #9	4 #9	—	—	—	—	4 #9	1 #9
TIES □	—	—	—	—	—	—	—	#4@12" □	—	—	—	—	#4@12" □	#4@12" □	#4@12" □	#4@12" □	—	—	—	—	#4@12" □	—
REMARKS	—	—	(✳)	—	—	(✳)	—	(✳)	—	—	—	—	(✳)	(✳)	(✳)	(✳)	($)	($)	($)	($)	(✳)	—

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COLUMN AND DRILLED FOOTING SCHEDULE

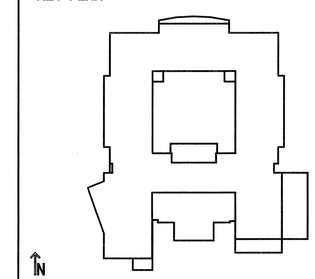
(RE: 1S1)
(EL. 0'–0" = SITE EL. 49.0')

NOTE A: CENTER ON GRADE BEAM –U.N.O.

LOCATION/MARK	U–5	U–6	U–7	V–21	V–22	W–9	W–14	W–18	W.1–16	X–5	Y–2	Y–3	Y–4	Y–5	Y–6	Y–7	Z–19	Z–21	Z–22
ELEV. @ TOP OF COLUMN	◄																		
CAP PLATE SIZE (INCHES)	◄																		
COLUMN SIZE (ASTM A992)	W8x58	W8x48	W8x35	W8x40	W8x35	W8x35	W8x31	W8x40	W8x40	W8x40	W8x31	W8x31	W8x31	W8x31	W8x35	W8x35	W8x31	W8x35	W8x31
BASE PLATE TYPE (RE: 1S7)	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–1'–2½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–3'–0½"
ELEV. @ TOP OF DR. FTG.	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–3'–4"	–3'–4"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–5'–2"	–2'–8"	–5'–2"
SHAFT DIAMETER (INCHES) (#)	42	36	36	30	36	30	30	36	30	30	24	36	30	30	30	36	24	30	30
SHAFT LENGTH (FEET)	50	45	40	35	40	35	35	35	35	40	35	40	35	35	40	40	35	35	35
VERTICAL REINF. (FULL HEIGHT)	1 #9	1 #9	1 #9	2 #9	1 #9	2 #9	1 #9	1 #9	1 #9	1 #9	2 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9
TIES □	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
REMARKS	—	—	—	(✳)	—	(✳)	—	—	—	—	(✳)	—	—	—	—	—	—	—	—

LOCATION/MARK	AA–4	AA–5	AA–6	BB–7	BB–8	BB–10	BB–11	BB–12	BB–13	BB–15	BB–16	BB–17	BB–19	CC–17	CC–19	DD–4	DD–5	DD–6	EE–8	EE–10	EE–11	EE–12
ELEV. @ TOP OF COLUMN	(SEE PLAN AND DETAILS)																					►
CAP PLATE SIZE (INCHES)	(½" x COLUMN SIZE – TYP. U.N.O.)																					►
COLUMN SIZE (ASTM A992)	W8x31	W8x35	W8x35	W8x31	W8x40	W8x35	W8x31	W8x31	W8x31	W8x31	W8x31	W8x35	W8x31	W8x35	W8x31	W8x31	W8x40	W8x40	W8x35	W8x35	W8x31	W8x31
BASE PLATE TYPE (RE: 1S7)	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–3'–0½"	–0'–6½"	–3'–0½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"
ELEV. @ TOP OF DR. FTG.	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–5'–2"	–2'–8"	–5'–2"	–2'–8"	–2'–8"	–2'–8"	–4'–8"	–2'–8"	–2'–8"	–2'–8"
SHAFT DIAMETER (INCHES) (#)	36	30	30	30	42	42	42	—	36	36	36	36	36	36	36	36	36	36	36	36	36	—
SHAFT LENGTH (FEET)	35	35	35	35	45	45	45	—	45	40	35	35	45	35	35	35	40	40	35	35	35	—
VERTICAL REINF. (FULL HEIGHT)	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	4 #9	—	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	4 #9	—
TIES □	—	—	—	—	—	—	#4@12" □	—	—	—	—	—	—	—	—	—	—	—	—	—	#4@12" □	—
REMARKS	—	—	—	—	—	—	(✳)	($)	—	—	—	—	—	—	—	—	—	—	—	—	(✳)	($)

KEY PLAN



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(✳) HOOK VERTICAL BARS @ TOP – BARS ON 12" GAGE EACH WAY @ 4 BAR PATTERN.
($) SHARES FOOTING WITH ADJACENT COLUMN.

NOTE A: CENTER ON GRADE BEAM –U.N.O.

COLUMN AND DRILLED FOOTING SCHEDULE

(RE: 1S1)
(EL. 0'–0" = SITE EL. 49.0')

(@) PROVIDE GALVANIZED EMBED PLATE ½"x14"x14" W/ 4–¾"øx12" HEADED STUDS @ TOP FOR BEAM.
(~) 4" CLEAR COVER.

LOCATION/MARK	EE–13	EE–15	FF–8	FF–10	FF–11	FF–12	FF–13	FF–15	FF–17	FF–19	GG–19	HH–4	HH–5	HH–6	HH–8	JJ–15	JJ–17	JJ.1–23.B	KK–8
ELEV. @ TOP OF COLUMN	◄																		
CAP PLATE SIZE (INCHES)	◄																		
COLUMN SIZE (ASTM A992)	W8x35	W8x35	W8x31	W8x35	W8x31	W8x31	W8x35	W8x31	W8x31	W8x31	W8x40	W8x31	W8x31	W8x31	W8x31	W8x31	W8x31	4"ø X–STR.	W8x31
BASE PLATE TYPE (RE: 1S7)	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	A	E	A
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–3'–0½"	–3'–0½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–3'–0½"	–3'–0½"	–0'–6½"
ELEV. @ TOP OF DR. FTG.	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–5'–2"	–5'–2"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–5'–2"	–5'–2"	–2'–8"
SHAFT DIAMETER (INCHES) (#)	36	36	30	36	36	—	36	30	36	36	36	30	36	36	36	36	30	24	30
SHAFT LENGTH (FEET)	35	35	35	35	35	—	35	35	35	35	35	35	40	40	35	35	35	35	35
VERTICAL REINF. (FULL HEIGHT)	1 #9	1 #9	1 #9	1 #9	4 #9	—	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9
TIES □	—	—	—	—	#4@12" □	—	—	—	—	—	—	—	—	—	—	—	—	—	—
REMARKS	—	—	—	—	(✳)	($)	—	—	—	—	—	—	—	—	—	—	—	—	—

LOCATION/MARK	KK–10	KK–11	KK–12	KK–13	KK–15	KK.1–23.A	KK.1–23.B	A	B	C	D	E	F	G	H	J	K	L	M	N	P	Q	R	S
ELEV. @ TOP OF COLUMN	(SEE PLAN AND DETAILS)																							►
CAP PLATE SIZE (INCHES)	(½" x COLUMN SIZE – TYP. U.N.O.)																							►
COLUMN SIZE (ASTM A992)	W8x31	W8x31	W8x31	W8x31	W8x31	4"ø X–STR.	8"øSTD.	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
BASE PLATE TYPE (RE: 1S7)	A	A	A	A	A	E	D	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
ELEV. @ BOT. OF BASE ℄	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–0'–6½"	–3'–0½"	–3'–0½"	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—
ELEV. @ TOP OF DR. FTG.	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–5'–2"	–5'–2"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–2'–8"	–5'–2"	–5'–2"	–5'–2"	–5'–2"	–5'–11"	–5'–11"	–5'–11"	–5'–11"	–4'–8"	–4'–0"	+4'–2"
SHAFT DIAMETER (INCHES) (#)	42	—	36	30	24	24	24	24	30	36	18	18	18	18	24	36	36	30	36	24	18	18	18	24
SHAFT LENGTH (FEET)	45	45	—	45	35	35	30	35	35	35	35	30	25	25	35	35	40	35	35	40	35	25	30	35
VERTICAL REINF. (FULL HEIGHT)	1 #9	4 #9	—	1 #9	1 #9	1 #9	2 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	1 #9	6 #9
TIES □	—	#4@12" □	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	—	#4@12" ○
REMARKS	—	(✳)	($)	—	—	—	(✳)	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	NOTE A	—	(@) (~)

USE OF ARCHITECT'S DRAWINGS, SPECIFICATIONS AND OTHER DOCUMENTS

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NO.	DATE	REVISION/SUBMISSION
1	11/12/04	FIRE MARSHAL AND C.P. REVISIONS

McKINLEY MIDDLE MAGNET SCHOOL

1550 EDDIE ROBINSON SR. DRIVE
BATON ROUGE, LOUISIANA 70802

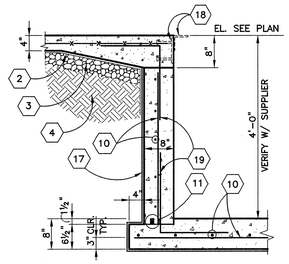
Robert M. Coleman Architects & Partners A.I.A.
3377 North Boulevard
Baton Rouge, Louisiana
70808
U.S.A.
Tel 225.387.4414
Fax 225.387.4693

COLUMN AND DRILLED FOOTING SCHEDULE

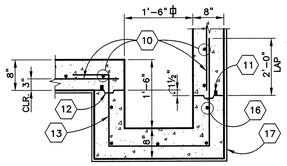
McKEE & DEVILLE CONSULTING ENGINEERS INC.
5131 PAYNE DRIVE BATON ROUGE, LA. 70809 TEL. (225)767-7841
JOB NO. 203079 ©2004

STATE OF LOUISIANA
DANNY J. DEVILLE
REG. NO. 20075
REGISTERED PROFESSIONAL ENGINEER IN
CIVIL ENGINEERING

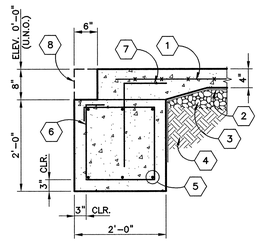
PROJECT NO.	03048
DRAWN BY	G.P.G.
CHECKED BY	D.J.D.
DATE	October 15, 2004
SHEET NO..	S6
	OF



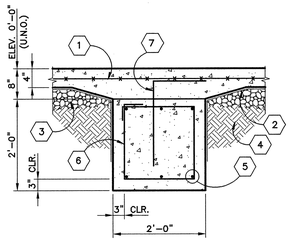
1 ELEVATOR PIT DETAIL
3/4" = 1'-0"



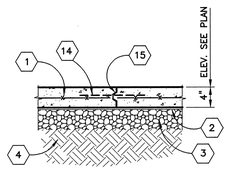
2 SUMP PIT DETAIL
3/4" = 1'-0"



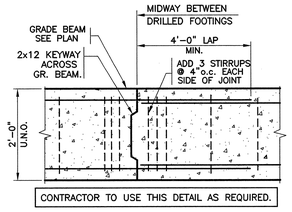
3 EXTERIOR GRADE BEAM
3/4" = 1'-0"



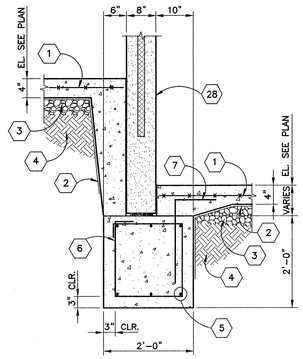
4 INTERIOR GRADE BEAM
3/4" = 1'-0"



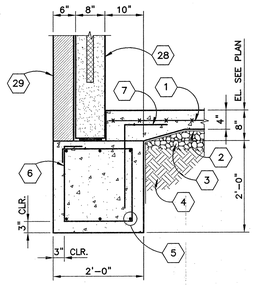
5 SLAB CONSTR. JOINT
1" = 1'-0"



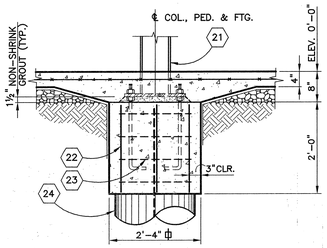
6 GRADE BEAM CONSTR. JOINT
3/4" = 1'-0"



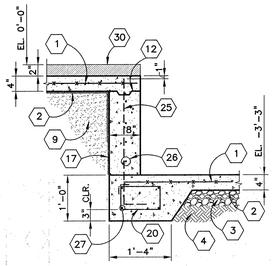
7 SECTION @ P/C WALL
3/4" = 1'-0"



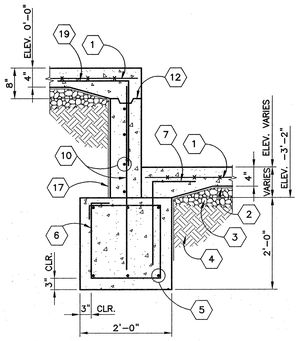
8 SECTION @ P/C WALL
3/4" = 1'-0"



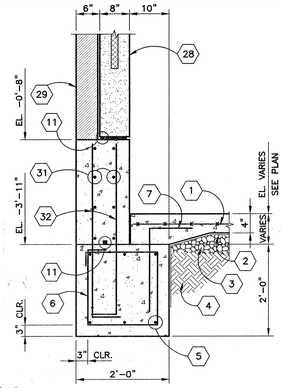
9 INTERIOR PEDESTAL DETAIL
3/4" = 1'-0"



10 SECTION AT STAGE
3/4" = 1'-0"



11 SECTION AT RAMP
3/4" = 1'-0"



12 GRADE BEAM @ AUDITORIUM
3/4" = 1'-0"

KEYNOTES

1. WWF 6x6 W2.9/W2.9 @ MID-DEPTH OF SLAB – PROVIDE IN FLAT SHEETS.
2. 10 MIL VAPOR RETARDER – VAPORBLOCK OR APPROVED EQUAL – SEE SPECIFICATIONS.
3. 4" THICK GRAVEL OR CRUSHED CONCRETE.
4. COMPACTED EARTH – SEE SPECIFICATIONS.
5. THREE #9 CONTINUOUS REBARS TOP & BOTTOM – HOOK TOP BARS @ ENDS OF GRADE BEAM. USE MINIMUM 48" LAP SPLICES. SPLICE TOP REBARS MID-WAY BETWEEN DRILLED FOOTINGS & BOTTOM REBARS @ DRILLED FOOTINGS WHEN SPLICES ARE REQUIRED.
6. #4 TIES @ 10" O.C. □
7. #3 DOWELS @ 12" O.C. 2' 2'
8. OMIT BRICK DAP WHERE REQUIRED PER ARCH. DRAWINGS.
9. COMPACTED SAND FILL AGAINST BACK OF WALL ONLY.
10. #5 @ 10" O.C. EACH WAY.
11. CONTINUOUS WATERSTOP RX BY CETCO OR APPROVED EQUAL.
12. CONTINUOUS 1.5"x3.5" FORMED KEYWAY.
13. 3 #4 EACH WAY.
14. ⅝"øx18" SMOOTH DOWEL @ 16" O.C. – GREASE ONE END.
15. CONTINUOUS GALVANIZED METAL KEYWAY.
16. TWO #4 □
17. WATERPROOFING – SEE ARCH.
18. SILL ANGLE & EMBED PLATES BY ELEVATOR SUPPLIER AS REQUIRED.
19. #4 DOWELS @ 10" O.C. 2' 2'
20. #3 TIES @ 12" O.C. □
21. COLUMN – SEE PLAN & SCHEDULE.
22. 8 #5 VERTICAL – TYPICAL @ COLUMNS.
23. 4 #4 TIES □ – TYPICAL @ COLUMNS.
24. DRILLED FOOTING – SEE PLAN & SCHEDULE.
25. #5 @ 8" O.C.
26. #5 CONTINUOUS @ 10" O.C.
27. TWO #5 CONTINUOUS TOP & BOTTOM.
28. 8" P/C PANEL – SHIM AND ATTACH TO GRADE BEAM PER PRECASTER SHOP DRAWINGS. PRECASTER TO DESIGN PRECAST PANEL AND ANCHORAGES FOR ALL GRAVITY, WIND & EARTHQUAKE LOADS PER IBC-2000 – SEE ARCH. FOR OTHER INFORMATION.
29. BRICK – ARCH.

15 GENERAL NOTES

A. APPLICABLE DESIGN CODES & MISCELLANEOUS

IBC 2000
AMERICAN CONCRETE INSTITUTE 318-98
AMERICAN INSTITUTE OF STEEL CONSTRUCTION DATED 1989

THE GEOTECHNICAL REPORT DATED JULY 14, 2004 AS PREPARED BY SOILS & FOUNDATION ENGINEERS, INC. MAY BE REVIEWED AT THE ARCHITECT'S OFFICE.

THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE SAFETY IN, OR ABOUT THE JOB SITE; IN CONTROL OF THE SAFETY OR ADEQUACY OF ANY EQUIPMENT, BUILDING COMPONENT, SCAFFOLDING, FORMS, OR OTHER WORK AIDS; OR SUPERINTENDING THE WORK.

B. CONCRETE AND GROUT

CONCRETE MIXING, HANDLING, PLACING AND CURING SHALL BE IN ACCORDANCE WITH ACI 301.

ALL CONCRETE SHALL BE NORMAL WEIGHT AND SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI AT 28 DAYS WITH A MINIMUM PORTLAND CEMENT CONTENT OF 5.7 BAGS PER CUBIC YARD.

ALL CONCRETE SHALL CONTAIN A MAXIMUM SLUMP OF 4" UNLESS THE CONTRACTOR USES A SUPERPLASTERCIZING ADMIXTURE.

FLY ASH IS NOT PERMITTED IN ANY CONCRETE FOR THIS PROJECT.

ALL GROUT SHALL BE NON-SHRINK GROUT.

ALL FLOOR DRAINS, DROPS, SLOPES, DAPS, CURBS, ETC. SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.

ALL EXPOSED SURFACES OF GRADE BEAMS, WALLS AND SLAB EDGES SHALL BE PLYWOOD FORMED AND COATED WITH A REPAIR MORTAR.

C. CONCRETE REINFORCEMENT

ALL REBARS SHALL BE GRADE 60 (FY = 60,000 PSI MINIMUM).

VAPOR RETARDER AT GROUND FLOOR SLABS TO BE 10 MIL ASTM E1745 CLASS C BARRIER. (VAPORBLOCK OR EQUAL).

HOOK ALL GRADE BEAM TOP BARS AT THE END OF THE GRADE BEAM.

PROVIDE 2 #6 3' 3' ONE TOP AND ONE BOTTOM AT THE OUTSIDE FACE OF ALL GRADE BEAM CORNERS.

PROVIDE 4 #6 3' 3' TWO TOP AND TWO BOTTOM AT ALL GRADE BEAM INTERSECTIONS.

D. STRUCTURAL STEEL

ALL STRUCTURAL STEEL SHALL BE ASTM A572-GRADE 50 OR ASTM A992. TUBES SHALL BE ASTM A500 GRADE B, AND PIPES SHALL BE ASTM A53 TYPE E OR S.

ALL STRUCTURAL STEEL SHALL BE FABRICATED, COATED AND ERECTED AS PER THE AISC SPECIFICATIONS – PRIMER SHALL BE COMPATIBLE WITH SPRAYED-ON FIREPROOFING.

ALL WELDS SHALL BE WITH E70XX ELECTRODES AND IN ACCORDANCE WITH AWS STANDARDS. MINIMUM FILLET WELD SIZE SHALL BE 1/4" – U.N.O.

ALL FRAMING CONNECTIONS SHALL BE STD. DOUBLE ANGLE CONNECTIONS MADE WITH THE MAXIMUM NUMBER OF ROWS OF 3/4" DIAMETER A325-N BOLTS FOR GIVEN BEAM DEPTH. USE BOLTS WITH TWIST-OFF TENSION INDICATORS.

THE CONTRACTOR SHALL ASSURE THAT THE STRUCTURE HAS BEEN ERECTED TRUE, AND SUITABLE TEMPORARY BRACING AND GUYS SHALL BE INSTALLED TO MAINTAIN SAID TRUENESS. THE STRUCTURAL STEEL FRAMEWORK SHALL BE BRACED OR GUYED UNTIL FINAL ERECTION IS COMPLETE AND DECKING AND PERMANENT BRACES HAVE BEEN ERECTED.

THE STEEL FABRICATOR SHALL PROVIDE AN ALLOWANCE IN HIS BASE BID FOR A TOTAL OF FOUR TONS OF ADDITIONAL ERECTED MISCELLANEOUS STEEL AS DEEMED NECESSARY BY STRUCTURAL ENGINEER. THIS ALLOWANCE SHALL COVER ALL MATERIALS, FABRICATION, PAINTING, DELIVERY AND ERECTION COSTS. THE EXACT SIZE AND QUANTITY OF STEEL MATERIALS SHALL BE SELECTED BY THE STRUCTURAL ENGINEER AS REQUIRED. ANY UNUSED PORTIONS OF THIS ALLOWANCE SHALL BE CREDITED BACK TO THE OWNER AT THE RATE OF $2,400.00 PER TON.

CONTRACTOR TO PROVIDE GALVANIZED STEEL LINTELS AS REQUIRED TO SUPPORT BRICK ABOVE ALL OPENINGS IN ACCORDANCE WITH THE FOLLOWING SCHEDULE (UNLESS NOTED OTHERWISE):

CLEAR OPENING (L)	ANGLE SIZE
0 < L ≤ 4'	4 X 4 X 1/4
4' < L ≤ 9'	6 X 4 X 3/8 LLV
9' < L ≤ 12'	7 X 4 X 3/8 LLV

THE USE OF REPRODUCTIONS OF THESE CONTRACT DRAWINGS BY ANY CONTRACTOR, SUB-CONTRACTOR, ERECTOR, FABRICATOR, OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS SIGNIFIES HIS ACCEPTANCE OF ALL INFORMATION SHOWN HEREON AS CORRECT AND OBLIGATES HIMSELF TO ANY JOB EXPENSE, REAL OR IMPLIED, ARISING DUE TO ANY ERRORS THAT MAY OCCUR HEREON.

ALL SHEAR CONNECTOR STUDS SHALL BE 3/4" DIAMETER BY 3½" LONG HEADED STUDS WITH SHEAR LOAD CAPACITY AS GIVEN IN TABLES 14.1 AND 14.2 OF THE AISC SPECIFICATIONS. ALL STUDS SHALL BE INSTALLED IN ACCORDANCE WITH THE AISC SPECIFICATIONS. INSTALLATION SHALL BE EQUALLY SPACED IN ROWS OF ONE OR TWO AS REQUIRED TO CONFORM TO MINIMUM AND MAXIMUM SPACINGS SET FORTH IN AISC SPECIFICATIONS.

E. DESIGN LOADS

(1) ROOF DESIGN LOADS

DEAD LOAD ---------- 25 PSF
LIVE LOAD ---------- 20 PSF

(2) FLOOR LOADS

DEAD LOAD ---------- 95 PSF
LIVE LOAD ---------- 80 PSF

(3) LATERAL DESIGN

105 MPH WIND PER IBC 2000
EARTHQUAKE PER IBC 2000

F. MASONRY

ALL MASONRY WORK SHALL BE DONE IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE, BIA, AND NCMA.

ALL CONCRETE FILL IN U-BLOCKS, PILASTERS AND CMU CELLS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS. PEA GRAVEL AGGREGATE SHALL BE USED.

U-BLOCK LINTELS SHALL BE PACED ABOVE ALL CMU OPENINGS, INCLUDING MECHANICAL PENETRATIONS, IN ACCORDANCE WITH THE FOLLOWING SCHEDULE (U.N.O.):

0 < CLEAR SPAN ≤ 4' ---------- 8" U-BLOCK W/#5 CONT. BOTTOM

4' < CLEAR SPAN ≤ 8' ---------- 16" U-BLOCK W/#6 CONT. BOTTOM

8' < CLEAR SPAN ≤ 15' ---------- 16" U-BLOCK W/#7 CONT. BOTTOM AND #5 CONT. TOP

16" U-BLOCK TO BEAR 16" PAST END OF OPENING AND
8" U-BLOCK TO BEAR 8" PAST END OF OPENING.

G. PREFABRICATED METAL TRUSSES

ALL PREFABRICATED ROOF TRUSSES AND OTHER ROOF FIELD FRAMED ELEMENTS SHALL BE SECURED TO ALL SUPPORTING MEMBERS BY MEANS OF WELDING OR OTHER APPROVED SYMMETRICAL MECHANICAL ATTACHMENT. TOUCH UP ALL FIELD WELDS WITH GALVANIZING REPAIR PAINT.

ALL PREFABRICATED ROOF TRUSSES SHALL BE FABRICATED, HANDLED, ERECTED, AND BRACED AS PER APPLICABLE CODES AND STANDARDS. TRUSS BOTTOM CHORDS MAY OR MAY NOT BE BRACED BY CEILING FRAMING – SEE ARCH. – PROVIDE TRUSS BRACING AS REQUIRED.

STRUCTURAL CALCULATIONS STAMPED BY A CIVIL ENGINEER LICENSED IN LOUISIANA SHALL BE PROVIDED FOR ALL PREFABRICATED COMPONENTS. ALSO PROVIDE, FOR APPROVAL, A TRUSS BRACING PLAN STAMPED BY A CIVIL ENGINEER LICENSED IN LOUISIANA. TRUSS BOTTOM CHORDS MAY OR MAY NOT BE BRACED BY CEILING FRAMING – SEE ARCH. – PROVIDE TRUSS BRACING AS REQUIRED.

MINIMUM DESIGN SUPERIMPOSED LOADS AND DEFLECTION CRITERIA FOR THE PREFABRICATED ROOF TRUSSES SHALL BE AS FOLLOWS (UNO):

WIND LOAD ---------- 105 MPH PER IBC 2000
TOP CHORD LIVE LOAD ---------- 20 PSF
TOP CHORD DEAD LOAD ---------- 10 PSF
BOTTOM CHORD DEAD LOAD ---------- 10 PSF
MAXIMUM LIVE LOAD DEFLECTION ---------- SPAN/420
MAXIMUM TOTAL LOAD DEFLECTION ---------- SPAN/360
MAXIMUM SPACING ---------- 48" O.C.
MINIMUM COMPONENT GAGE ---------- 20 GAGE

METAL TRUSS SUPPLIER TO DESIGN AND PROVIDE ALL HEADERS AND METAL CONNECTORS FOR TRUSS TO TRUSS CONNECTIONS.

MECHANICAL PIPE HANGERS SHALL BE NO MORE THAN 12" AWAY FROM A TRUSS PANEL POINT AND SHALL BE SPACED @ 4' O.C. MAXIMUM. TRUSS DESIGNER TO INCLUDE PIPE LOADS IN TRUSS DESIGN. SEE MECHANICAL DRAWINGS. CONTRACTOR TO PROVIDE CONCENTRICALLY LOADED U-STRAPS FOR ATTACHMENT OF HANGER RODS TO TRUSSES. DRILLING HOLES THRU TRUSS CHORDS IS NOT ALLOWED.

H. METAL DECKING

ALL METAL DECK SHALL BE FABRICATED AND ERECTED AS PER THE STEEL DECK INSTITUTE'S STANDARDS AND THE MANUFACTURERS SPECIFICATIONS.

ALL ROOF METAL DECK SHALL BE GALVANIZED TYPE 1.5B-22 GAGE G60 DECK ON A THREE SPAN CONDITION MINIMUM. ATTACH TO ALL SUPPORTS WITH #12 SCREWS ON A 36/4 PATTERN.

ALL FLOOR METAL DECK SHALL BE GALVANIZED 2 VLI – 20 GA. G60 COMPOSITE DECK ON A THREE SPAN CONDITION MINIMUM. ATTACH TO ALL SUPPORTS WITH 5/8" DIAMETER PUDDLE WELDS ON A 36/4 PATTERN. FASTEN DECK TO PERIMETER EDGE ANGLE WITH 5/8" DIAMETER PUDDLE WELDS AT 12" O.C. MAXIMUM. SHEAR STUDS MAY BE USED AS DECK ATTACHMENTS. DECK SUPPLIER SHALL PROVIDE ALL REQUIRED DECK CLOSURES, GIRDER FILLERS, COLUMN COVERS, ETC. AS NEEDED FOR A COMPLETE DECKING INSTALLATION. TOUCH UP ALL DECK WELDS AND SHEAR STUD WELDS WITH GALVANIZING REPAIR PAINT.

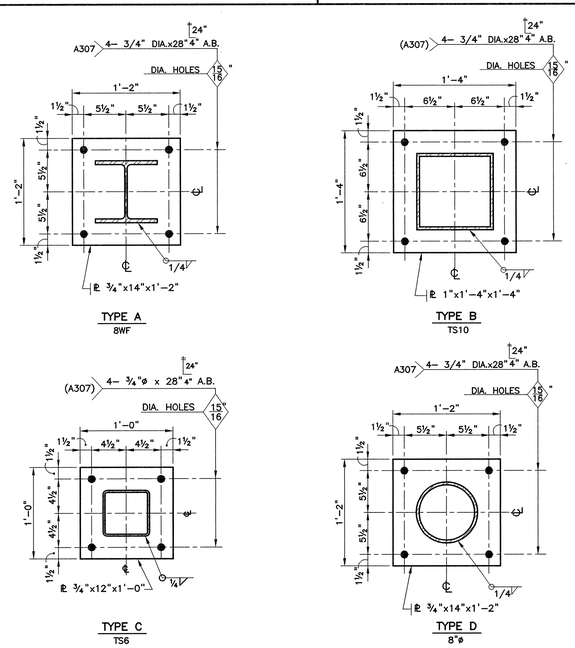
I. BAR JOIST

ALL JOISTS SHALL BE CONNECTED TO SUPPORTS BY WELDING AS PER THE STEEL JOIST INSTITUTE REQUIREMENTS. FABRICATION, COATING AND ERECTING OF ALL JOISTS SHALL BE IN ACCORDANCE WITH SJI SPECIFICATIONS. DESIGN ROOF JOISTS FOR NET UPLIFT OF 20 PSF.

BRIDGING FOR BAR JOISTS SHALL BE AS REQUIRED BY SJI – UNLESS NOTED OTHERWISE ON PLANS.

TWO BAYS OF X-BRACING, (ANGLE 1-3/4 x 1-3/4 X 3/16) SHALL BE PROVIDED AT THE END OF ALL JOIST BRIDGING ROWS UNLESS NOTED OTHERWISE ON PLANS. BRIDGING SHALL BE PROVIDED BY JOIST SUPPLIER.

ALL EQUIPMENT & CATWALKS SUSPENDED FROM OR RESTED ON BAR JOIST AT POINT LOCATIONS SHALL BE SO DONE AT A PANEL POINT LOCATION OF THE JOIST. THE JOIST SUPPLIER SHALL PROVIDE SUCH PANEL POINTS AT THESE LOCATIONS AS WELL AS WHERE BRACE ANGLES ARE CONNECTED TO JOIST BOTTOM CHORDS.



13 BASE PLATE DETAILS
1 1/2" = 1'-0" (RE: 1S6)

J. PRECAST CONCRETE PANELS

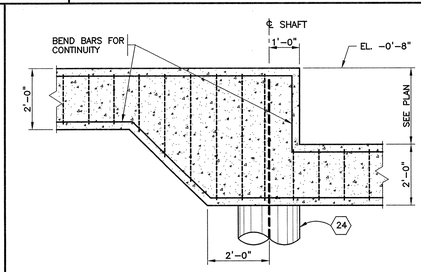
ALL PRECAST CONCRETE PANELS SHALL BE DESIGNED FOR ALL APPROPRIATE GRAVITY, WIND AND EARTHQUAKE LOADS PER IBC-2000. LOAD BEARING WALLS SUPPORTING ROOF LOADS SHALL BE DESIGNED FOR A ROOF DEAD LOAD OF 25 PSF, LIVE LOAD OF 20 PSF, AND NET WIND UPLIFT LOAD OF 20 PSF.

SUPPLIER SHALL DESIGN AND PROVIDE ALL CONNECTION HARDWARE TO ATTACH PANELS TO FOUNDATION AND OTHER STRUCTURAL ELEMENTS.

FINISH AND INSULATION REQUIREMENTS FOR PANELS SHALL BE PER ARCHITECTURAL DETAILS AND SPECIFICATIONS.

K. STEEL STAIRS

ALL STAIRS SHALL BE PRE-ENGINEERED STEEL STAIR SYSTEMS DESIGNED & SUPPLIED BY THE STAIR SUPPLIER. STRUCTURAL CALCULATIONS & SHOP DRAWINGS SHALL BE PROVIDED TO ARCHITECT FOR REVIEW. PROVIDE ALL NECESSARY COLUMNS, HANGER RODS, ETC. STAIRS SHALL CONFORM TO ARCHITECTS DIMENSIONAL CONSTRAINTS AND SHALL BE DESIGNED FOR A LIVE LOAD OF 100 PSF.

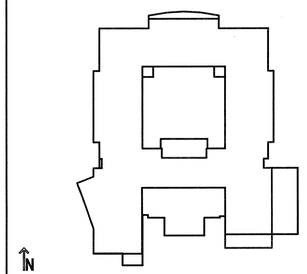


14 GRADE BEAM STEP DETAIL
1/2" = 1'-0" (GBS)

KEYNOTES
(CONTINUED)

30. FLOORING – SEE ARCH.
31. #4 @ 10" O.C.E.F.
32. #5 @ 8" O.C.E.F. 58" 10"

KEY PLAN



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1	11/12/04	FIRE MARSHAL AND C.P. REVISIONS

McKINLEY MIDDLE MAGNET SCHOOL

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DETAILS AND GENERAL NOTES

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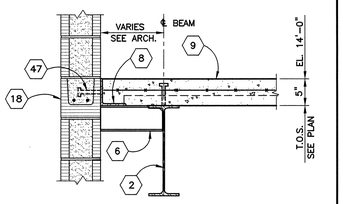
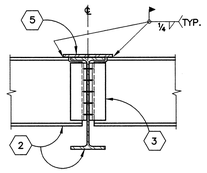
JOB NO. 203079 © 2004

STATE OF LOUISIANA
DANNY J. DEVILLE
REG. NO. 20075
REGISTERED PROFESSIONAL ENGINEER IN CIVIL ENGINEERING

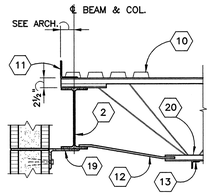
PROJECT NO.	03048
DRAWN BY	G.P.G.
CHECKED BY	D.J.D.
DATE	October 15, 2004
SHEET NO..	S7
	OF



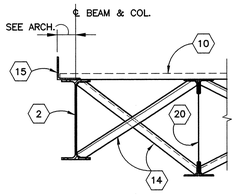
1 EXTERIOR FLOOR SECTION
1" = 1'-0"



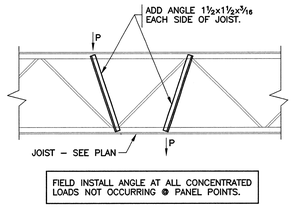
2 BEAM TOP PLATE CONNECTION
3/4" = 1'-0" (TYPICAL)



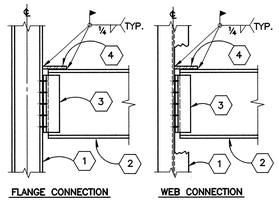
3 EXTERIOR ROOF SECTION
3/4" = 1'-0"



4 EXTERIOR ROOF SECTION
1" = 1'-0"



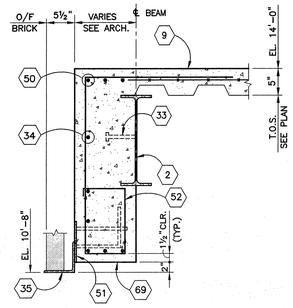
5 ADDED JOIST VERTICAL @ CONCENTRATED LOADS
1" = 1'-0"



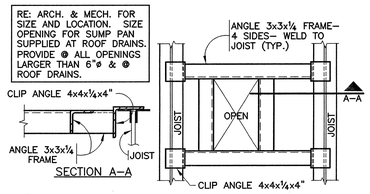
6 BEAM/COLUMN CONNECTION
3/4" = 1'-0" (TYPICAL)

KEYNOTES

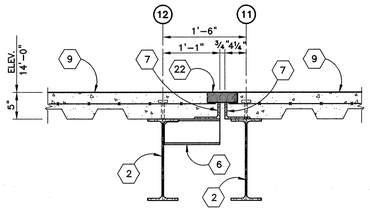
1. COLUMN – SEE PLAN & SCHEDULE.
2. BEAM – SEE PLAN.
3. STANDARD DOUBLE ANGLE FRAMING CONNECTION W/ THE MAXIMUM NUMBER OF ROWS OF ¾"ø A325–N BOLTS. USE BOLTS CONTAINING TWIST–OFF TENSION INDICATORS (TYP.)
4. PLATE ¼"x4"x4" @ EACH BEAM.
5. PLATE ¼"x4"x1'–2" @ EACH BEAM LOCATION.
6. C5x6.7 @ 2' O.C. – WELD TO BEAM ALL AROUND – THIS CHANNEL REQUIRED ONLY WHEN DIMENSION FROM ℄ BEAM TO EDGE OF ANGLE EXCEEDS 5½".
7. CONTINUOUS ANGLE 5x3½x⅜ SLV – WELD TO CHANNEL OUTLOOKER/ BEAM ALL AROUND AS APPLICABLE.
8. CONTINUOUS ANGLE 5x5x⅜– WELD TO BEAM W/ 3" OF ¼" FILLET WELD @ 12" O.C. FRONT & BACK SIDE OR CHANNEL OUTLOOKER ALL AROUND AS APPLICABLE.
9. 5"–3500 PSI NORMAL WEIGHT CONCRETE W/ WWF 6x6 W2.9/W2.9 ON GALVANIZED 2VLI–20 GAGE COMPOSITE METAL DECK. (TYP.)
10. GALV. 1.5B–22 GAGE G60 METAL DECK – SEE GENERAL NOTES AND AND SPECIFICATIONS. ATTACH TO ALL SUPPORTS W/ #12 TEK SCREWS ON A 36/4 PATTERN. USE ACOUSTICAL DECK OVER GYM AS SPECIFIED ON PLAN.
11. CONTINUOUS ANGLE 4x4x¼ – WELD TO JOIST ALL AROUND.
12. TWO ¾"ø RODS @ EVERY OTHER JOIST – FIELD WELD ALL AROUND. INSTALL AFTER ALL DEAD LOAD IS ON ROOF. AN ANGLE 2½x2½x¼ MAY BE USED IN LIEU OF DOUBLE RODS.
13. CONTINUOUS ANGLE 2x2x5/16 – WELD TO JOIST ALL AROUND.
14. TWO BAYS OF X–BRACE ANGLE 1¾x1¾x3/16 AT ENDS OF ALL BRIDGING ROWS BY JOIST SUPPLIER – WELD ALL AROUND.
15. CONTINUOUS ANGLE 4x4x¼ – WELD TO BEAM W/ 2" OF ¼" FILLET WELD @ 12" O.C. – TOP & BOTTOM SIDE.
16. PLATE ½"x6"x8" W/ 2–⅝"øx8" HEADED STUDS ON 5" GAGE AT EVERY JOIST – WELD TO JOIST.
17. 8" P/C CONCRETE PANEL – PRECASTER TO DESIGN FOR ALL GRAVITY, WIND & EARTHQUAKE LOADS PER IBC 2000 – SEE ARCH. FOR OTHER INFORMATION.
18. CONTINUOUS 8" DEEP U–BLOCK W/ 2 #5 CONTINUOUS IN CONCRETE FILL.
19. ANGLE 6x6x⅜x4" LONG @ 4' O.C. – BOLT TO CONCRETE FILLED CMU W/ ⅝"ø x6" EXPANSION BOLT & WELD TO BEAM. PROVIDE 13/16"x2" VERTICAL SLOTTED HOLE IN ANGLE.
20. JOIST – SEE PLAN.
21. GALVANIZED SYMMETRICAL U–SADDLE OR APPROVED EQUAL WELDED TO BEAM & SCREW TO TRUSS (TYPICAL ALL TRUSSES @ ALL BEAM SUPPORTS).



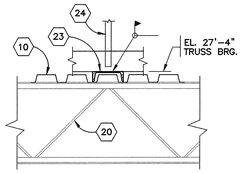
7 SECTION @ HUNG BRICK
1" = 1'-0"



8 ROOF OPENING FRAME
1½" = 1'-0"



9 FLOOR SECTION @ EXPANSION JOINT
1" = 1'-0" (@ CORRIDOR)

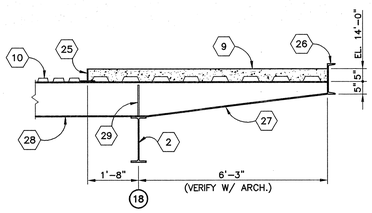


10 PRE–FAB TRUSS TO JOIST CONN.
1" = 1'-0" (TYPICAL)

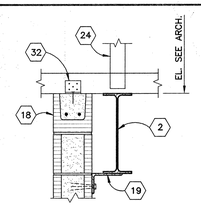
KEYNOTES

(CONTINUED)

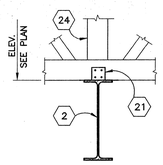
22. BLOCKOUT FOR EXPANSION JOINT COVER – SEE ARCH.
23. PIECE OF C6x8.2 x 8" LONG @ 8' O.C. MAX. – WELD TO JOIST – LOCATE @ JOIST TOP CHORD PANEL POINT.
24. PRE–FAB METAL TRUSS @ 4' O.C. – TRUSS MUST ALIGN WITH JOIST WHERE APPLICABLE – WELD TO EACH SUPPORT.
25. CONTINUOUS WT 5x11 – WELD TO BEAM ALL AROUND.
26. CONTINUOUS C12x20.7 – WELD TO EACH BEAM.
27. PLATE ⅜"x6" – WELD TO W14 WEB W/ CONTINUOUS ¼" FILLET WELD EACH SIDE OF WEB.
28. W14x22 – CUT BOTTOM FLANGE & ADD PLATE – SEE ARCH.
29. ¼" STIFFENER PLATE EACH SIDE OF WEB.
30. GALVANIZED SYMMETRICAL U–SADDLE OR APPROVED EQUAL WELDED TO EMBED PLATE & SCREW TO TRUSS (TYPICAL @ ALL TRUSSES @ ALL SUPPORTS).
31. EMBED PLATE ⅜"x4"x8" W/ 2–⅝"øx4" HEADED STUDS ON 5" GAGE. PROVIDE @ EACH METAL TRUSS LOCATION @ ALL BEARING WALLS.
32. 16 GAGE CLIP ANGLE EACH SIDE OF EACH TRUSS – FASTEN TO BLOCK & SCREW TO TRUSS.
33. ⅝"øx6" HEADED STUD @ 12" O.C. – WELD TO BEAM.
34. #5 @ 10" O.C. CONTINUOUS.
35. CONTINUOUS GALVANIZED ANGLE 5x5x⅜ – WELD TO EACH EMBED PLATE ALL AROUND W/ ¼" FILLET WELD.
36. 5" EXTENDED END – MAXIMUM ¼" DEFLECTION @ END.
37. PIECE OF W21x44 x 8" LONG @ EACH SLOPING JOIST – WELD TO LOWER BEAM ALL AROUND.
38. W6x9 @ ±6' O.C. – WELD TO JOIST & EMBED PLATE – SEE PLAN.
39. EMBED PLATE BY PRECAST SUPPLIER @ 4' O.C.
40. CLIP ANGLE 4x4x⅜x8" LONG @ EACH HANGER – WELD ALL AROUND.
41. CONTINUOUS ANGLE 3x3x¼ – WELD TO EACH VERTICAL ANGLE.
42. CONTINUOUS PLATE ⅜"x6" – WELD TO CHANNEL & EACH HANGER ANGLE.
43. HANGER ANGLE 4x4x¼ @ 4' O.C.± ALIGN WITH BOTTOM CHORD PANEL POINT OF JOIST.
44. 1"x3/16" GW–100 STEEL GRATING SPANNING TO & WELDED TO CHANNELS.
45. CROSS ANGLE 2½x2½x¼ @ 4' O.C. – WELD TO CHANNELS.
46. CONTINUOUS C8x11.5 – WELD TO EACH VERTICAL ANGLE.
47. ⅝"øx5" LONG HEADED STUDS @ 32" O.C. – WELD TO EDGE ANGLE.
48. CONTINUOUS ANGLE 6x4x⅜ LLV – WELD TO EACH JOIST.
49. TWO BAYS OF X–BRACE ANGLE 3x3x¼ @ 4' O.C.
50. 3 #9 T&B CONTINUOUS.
51. EMBED PLATE ⅜"x8"x8" W/ 4–½"ø BENT ANCHORS @ 2' O.C.
52. #4 @ 12" O.C.
53. SPECIAL JOIST SEAT AS REQUIRED.
54. CLIP ANGLE 8x4x⅜x4" SLV @ 2' O.C. – WELD TO JOIST & EMBED ALL AROUND.
55. 3"–3500 PSI NORMAL WT. CONCRETE W/ WWF 6x6 W2.9/W2.9 ON GALVANIZED 0.6C–24 GAGE METAL DECK (@ MEZZANINE ONLY).
56. EMBED PLATE ⅜"x7"x6" W/ 2–⅝"øx4" HEADED STUDS ON 3" GAGE AT EACH JOIST. WELD JOIST TO EMBED.
57. CONTINUOUS ANGLE 3x3x¼ – WELD TO JOIST ALL AROUND – PROVIDE AROUND PERIMETER OF MEZZANINE.
58. CONTINUOUS ANGLE 4x4x⅜ WELDED TO BEAM W/ 3" OF ¼" FILLET WELD @ 12" O.C. T&B SIDE AND TO EACH VERTICAL CHANNEL.
59. CONTINUOUS CONCRETE CURB FOR BRICK – SEE ARCH. – REINFORCE W/ 2 #4.
60. C6x8.2 @ 4' O.C. – WELD TO STRUCTURE ALL AROUND.
61. CONTINUOUS GALVANIZED ANGLE 6x6x⅜ – WELD TO EACH HANGER ALL AROUND.
62. CLIP ANGLE 4x4x⅜x6" @ EACH HANGER – WELD ALL AROUND.
63. ANGLE 4x4x⅜ @ EACH HANGER – WELD TO BEAMS ALL AROUND.
64. ANGLE 4x4x⅜ @ EACH HANGER – WELD ALL AROUND.
65. ⅜" END PLATE.
66. 1–⅝"øx4" EXPANSION BOLT TO CONCRETE FILLED CMU CELL.
67. CONTINUOUS ANGLE 5x5x⅜ – WELD TO BEAM W/ 2" OF ¼" FILLET WELD @ 12" O.C. – TOP & BOTTOM SIDE.
68. FLEXIBLE MATERIAL – SEE ARCH.
69. CONCRETE BEAM TO BEAR ON CONCRETE FILLED PILASTER @ ENDS.
70. 1 #5 IN CONCRETE FILLED CELL FULL HEIGHT @ EACH ANGLE STRUT.



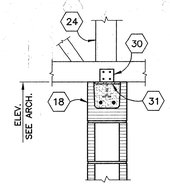
11 SECTION @ BRIDGE
1/2" = 1'-0"



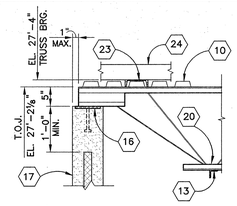
12 EXTERIOR ROOF SECTION
1" = 1'-0" (TYPICAL CONCEPT)



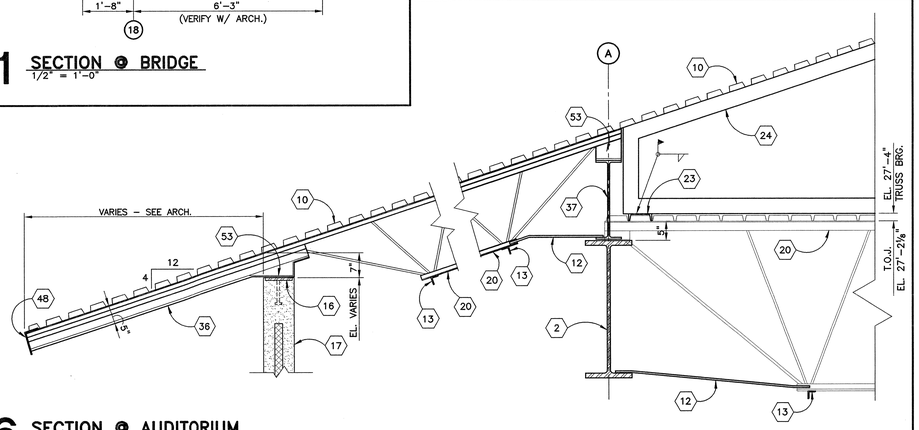
13 TRUSS TO BEAM CONN.
1" = 1'-0" (TYPICAL)



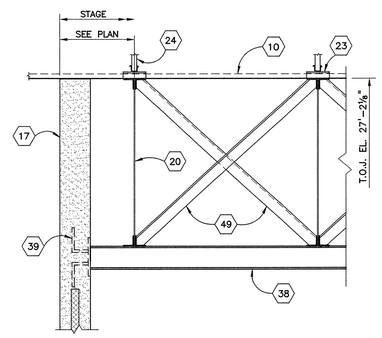
14 TRUSS TO BRG. WALL CONN.
1" = 1'-0" (TYPICAL – U.N.O.)



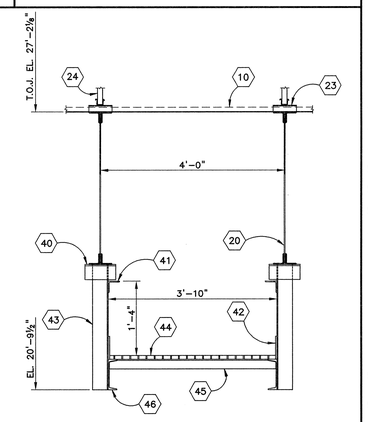
15 JOIST TO P/C WALL CONN.
3/4" = 1'-0" (TYPICAL CONCEPT)



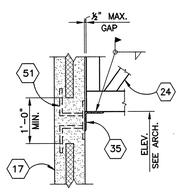
16 SECTION @ AUDITORIUM
3/4" = 1'-0"



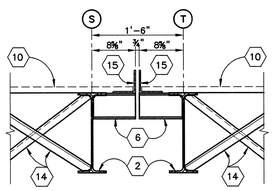
17 SECTION @ PROSCENIUM WALL
3/4" = 1'-0"



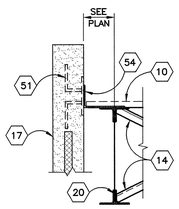
18 SECTION @ CATWALK
3/4" = 1'-0"



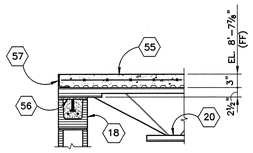
19 TRUSS CONNECTION TO P/C WALL
3/4" = 1'-0" (TYPICAL)



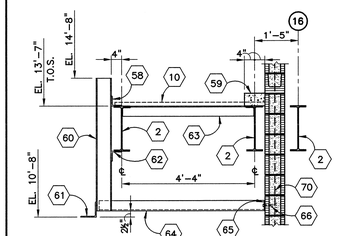
20 SECTION @ ROOF EXPANSION JT.
1" = 1'-0"



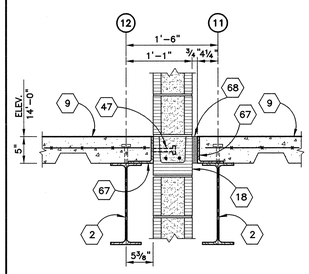
21 ROOF SECTION @ P/C WALL
1" = 1'-0"



22 SECTION @ MEZZANINE
3/4" = 1'-0"

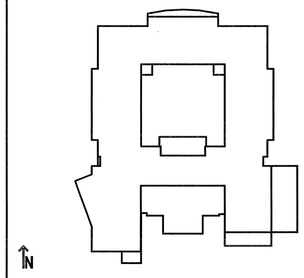


23 SECTION @ BRICK HANGER
1/2" = 1'-0"



24 FLOOR SECTION @ EXPANSION JOINT
1" = 1'-0" (@ CLASSROOMS)

KEY PLAN



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JOB NO. 203079 @ 2004

STATE OF LOUISIANA — DANNY K. DEVILLE — REG. NO. 20075 — REGISTERED PROFESSIONAL ENGINEER IN CIVIL ENGINEERING

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DRAWN BY	G.P.G.
CHECKED BY	D.J.D.
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